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December 16, 1985

Smith, Hinchman & Grylls Associates, Inc.
455 W. Fort Street
Detroit, Michigan 48226

Attention: E. Medling

Subject: GS-11B-19066, Structural Steel Delay Claim, Centex P-61, CE 35

Gentlemen:

In connection with the above subject and the ongoing discussions regarding same, we have received several pieces of correspondence from the contractor. These items include:

- 1.) Centex letter of December 11, 1985 with 10 pages attached (Bristol Steel and Palmetto Steel)
- 2.) Centex letter of December 5, 1985 with 4 pages attached (Bristol Steel)
- 3.) Bristol's 3 page "Delay Bar Chart"

We are forwarding these items herewith for your review and any appropriate rebuttal, comments, etc.

Sincerely,


Arthur J. Canfield
Project Manager
Headquarters Expansion Project

AJC:pb

cc: Gary Lee

Enclosures

STAT

ANG
12-6



CENTEX CONSTRUCTION COMPANY, INC.

POST OFFICE BOX 427 • MERRIFIELD, VA. 22116 • TELEPHONE (703) 698-6886

PLEASE ADDRESS REPLY TO:

December 5, 1985

P.O. BOX 6510
McLEAN, VA 22106-6510
Phone: (703) 448-6810

Arthur J. Carlucci, Project Manager
New Building Project Field Office
Room 3E40, Headquarters Building
Washington, D.C. 20505

Re: CIA Headquarters Expansion
Bid Package 2
Langley, Virginia
Contract No. GS-11B-19066

Subj: Centex Claim P-61
Structural Steel and Deck
Delay Claim

CR#35

Gentlemen:

On November 26, 1985 we met at the GSA's project office to review the subject claim with the concerned parties. A follow-up meeting with the GSA has been set for Tuesday, December 10, 1985 at 9:30 a.m. to discuss Centex Construction Company's position.

Attached for your review and information is a copy of Bristol Steel and Iron Works letter of December 2, 1985 summarizing our position as affected by SH&G's shop drawing review of the CSI deck submittals.

Very truly yours,

CENTEX CONSTRUCTION COMPANY, INC.

Joseph A. Pius
Assistant Project Manager

JAP:sap

Enclosure

cc: Merrifield Office
Proposal File P-61

RICHMOND DIVISION
1000 East Fourth Street
P.O. Box 27532
Richmond, Virginia 23261
(804) 233-7691 Telex 827350

Bristol Steel

AND IRON WORKS, INC.

December 2, 1985

①

Centex Construction Co., Inc.
P. O. Box 6510
McLean, Virginia 22106-6510

ATTN: Mr. John Milhausen

Re: CIA Headquarters Expansion
Langley, Virginia
BS&IW Contract 1814

Subj: Contract Extension Meeting
of 11/26/85

Dear Mr. Milhausen:

At the conclusion of the subject meeting, Mr. Roy Daniel stated that we wanted to think about the statements made during the meeting and to have another meeting with the GSA, after we had completed our review. The subject meeting had two distinct segments, the structural steel drawings resubmitted for approval and the deck drawings resubmitted for approval. Contained herein is our response to the Architect's position on the deck drawings, our response to his position on the structural drawings will be forwarded shortly.

The Architect stated during this meeting, that his firm had made errors in reviewing the deck drawings. We feel that the red lines the Architect noted on the drawings was not sufficient cause for resubmittal. If you review our letter of August 19, 1985, you will find drawing 4 had more red line corrections on it than most of the other sheets. Submittal one and submittal two of drawing 4 had a similar number of "corrections", submittal two received approval code 2 and submittal one received approval code 3. Additionally, a substantial number of corrections were in the length of the sheets of deck. Lengths are solely the responsibility of the deck manufacturer, the Architect's only responsibility regarding lengths is to ascertain that the column to column dimensions are correct, not the lengths of individual pieces of deck.

The Architect stated that the primary reason for the resubmittal of the deck drawings was the lack of a stud layout drawing and the stud count shown on the drawings. As we have stated previously, the stud count and layout are provided us as a courtesy by the deck manufacturer. The manufacture and installation of the deck and accessories is independent of the studs. If you will refer to Section 05320, Page 3, Lines 83 to 85 of the job specifications (attached), you will find "decking shall be installed independent of stud shear connectors. Welding of shear connectors shall not be used as a method of fastening the decking." This clause of the specifications clearly directs us to install these items independently, therefore the deck manufacture and installation is independent of the stud installation. At the very least the deck drawings should have been approved as noted with a notation by the Architect stating - stud layout and stud count to be resubmitted for approval, deck manufacture and installation approved as noted. Had the Architect done this no delay would have occurred.



DEPENDABLE STRUCTURAL STEEL SERVICE SINCE 1908


Bristol Steel
AND IRON WORKS INC.

Centex Construction Co., Inc.
December 2, 1985
Page ②

The previous would have prevented delay had the stud layout been a requirement of our deck submittal. However, the specifications do not refer to studs in describing the deck drawing submittals. In Section 05300, Lines 8 to 10, and Section 05320, Lines 28 to 30 (attached), we find "Submit shop drawings indicating layout of the deck, sections, reinforcing for openings larger than 6 inches, closures, accessories, and other details of fabrication, installation and welding procedures." The studs are not mentioned in this description. The only area of the drawing description that might include the studs is the item "...accessories ...", but if you refer to Section 05320, Lines 93 to 98 of the specifications (attached) you will see that the studs are not considered a part of the accessories. The final clause of the drawing specifications "... other details of fabrication, installation, and welding procedures." refers only to the aforementioned items, if inclusion of the studs was intended, it would have been clearly stated.

We normally obtain our stud count from the contract drawings and are obligated to provide the studs specified therein. Therefore, we continue to maintain that the resubmittal of the deck layout drawings for approval was unnecessary, time consuming and resulted in delays in the project, in accordance with our previous total of days of delay incurred.

Sincerely,



Chester A. Bush
Contracting Engineer

CAB/ce

cc: Mr. Roy Daniel*
Mr. Carl Soles *

*w/attachment

(3)

SH&G 13155 BP-2
CENTRAL INTELLIGENCE AGENCY
HEADQUARTERS EXPANSION
BID PACKAGE 2
HEADQUARTERS BLDG & VISITORS CENTER

SECTION 05320 PAGE 1
DATE 03/13/84 TIME 17.633
METAL FLOOR DECK

ARTICLE	INDEX	LINE
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2. SUBMITTALS		26
3. DECK REQUIREMENTS		38
4. DECK TYPE AND MANUFACTURE		47
5. MATERIALS		65
6. ERECTION		79
***END OF INDEX		

1. GENERAL PROVISIONS

- 1) 1)
- 2) 2) RELATED WORK SPECIFIED UNDER OTHER SECTIONS:
- 3) 3) SHEET METAL FLASHING AND TRIM WORK - 07600.
- 4) 4) PLATE CLOSURES (3/16-INCH AND THICKER).
- 5) 5) HOT-ROLLED SHAPE REINFORCING.
- 6) 6) PAINTING - 09901.
- 7) 7) REFERENCE SPECIFICATIONS
- 8) 8) THE WORK SHALL COMPLY WITH APPLICABLE REQUIREMENTS OF THE FOLLOWING:
- 9) 9) AMERICAN IRON AND STEEL INSTITUTE (AISI) - SPECIFICATION FOR THE
- 10) 10) DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
- 11) 11) AMERICAN WELDING SOCIETY (AWS) - STRUCTURAL WELDING CODE, AWS D1.1.
- 12) 12) UNDERWRITERS LABORATORIES INC. (UL) - FIRE RESISTANCE INDEX.
- 13) 13) EXAMINATION AND ACCEPTANCE OF WORK IN PLACE
- 14) 14) EXAMINE WORK IN PLACE ON WHICH THIS WORK IS DEPENDENT. DEFECTS WHICH
- 15) 15) MAY INFLUENCE SATISFACTORY COMPLETION AND PERFORMANCE OF THIS WORK
- 16) 16) SHALL BE CORRECTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE
- 17) 17) APPLICABLE SECTION OF WORK PRIOR TO COMMENCEMENT OF THE WORK.
- 18) 18) COMMENCEMENT SHALL BE CONSTRUED AS WORK IN PLACE BEING ACCEPTABLE FOR
- 19) 19) SATISFYING THE REQUIREMENTS OF THIS SECTION.
- 20) 20) FIELD MEASUREMENTS
- 21) 21) TAKE FIELD MEASUREMENTS TO VERIFY AND SUPPLEMENT DIMENSIONS INDICATED.
- 22) 22) STORAGE
- 23) 23) UPON DELIVERY, ALL DECK SHALL BE STORED OFF THE GROUND WITH ONE END
- 24) 24) ELEVATED TO PROVIDE DRAINAGE AND SHALL BE PROTECTED WITH A WATERPROOF
- 25) 25) COVERING WHICH ALLOWS AIR CIRCULATION.

2. SUBMITTALS

27) SHOP DRAWINGS

- 28) 28) SUBMIT SHOP DRAWINGS INDICATING LAYOUT OF THE DECK SECTIONS,
- 29) 29) REINFORCING FOR OPENINGS LARGER THAN 6 INCHES, CLOSURES, ACCESSORIES
- 30) 30) AND OTHER DETAILS OF FABRICATION, INSTALLATION AND WELDING PROCEDURES.

31) UL DESIGN NO.

(4)

SH86 13155 BP-2
CENTRAL INTELLIGENCE AGENCY
HEADQUARTERS EXPANSION
BID PACKAGE 2
HEADQUARTERS BLDG & VISITORS CENTER

SECTION 05320 PAGE 3
DATE 03/13/84 TIME 17.633
METAL FLOOR DECK

- 7 74) DECK UNITS - 20 GAGE.
- 7 75) CLOSURES AND REINFORCING - 16 GAGE.
- 2 76) [REDACTED]
- 3 77) [REDACTED]
- 3 78) [REDACTED]
- 1 79) [REDACTED]
- 2 80) GENERAL
- 3 81) INSTALL DECK UNITS AND ACCESSORIES IN ACCORDANCE WITH THE
- 3 82) MANUFACTURER'S INSTRUCTIONS AND APPROVED SHOP DRAWINGS.
- 3 83) [REDACTED]
- 3 84) [REDACTED]
- 3 85) [REDACTED]
- 2 86) DECK UNIT FASTENING
- 3 87) FASTEN DECK UNITS TO SUPPORTING STEEL STRUCTURE AT ENDS AND AT
- 3 88) INTERMEDIATE SUPPORTS BY WELDS NOT LESS THAN 3/4 INCH IN LENGTH OR
- 3 89) DIAMETER AT NOT MORE THAN 12-INCH SPACING. ALL WELDS SHALL BE
- 3 90) ACCESSIBLE FOR TOUCH-UP PAINTING.
- 3 91) ENGAGE AND BUTT-PUNCH SIDE LAPS AT A MAXIMUM SPACING OF 3 FEET.
- 3 92) WELDING OF SIDE LAPS WILL NOT BE PERMITTED.
- 2 93) ACCESSORIES
- 3 94) PROVIDE SHEET STEEL CLOSURES AT ENDS AND SIDES OF METAL FLOOR DECK.
- 4 95) END CLOSURES SHALL BE FASTENED TO UNITS WITH TACK WELDS OR SHEET
- 4 96) METAL SCREWS.
- 4 97) SIDE CLOSURES SHALL BE TACK-WELDED TO STRUCTURE AT 3-FOOT SPACING.
- 3 98) PROVIDE SHEET STEEL CLOSURES AROUND COLUMNS.
- 2 99) [REDACTED]
- 3 100) [REDACTED]
- 3 101) [REDACTED]
- 2 102) OPENINGS
- 3 103) OPENINGS LARGER THAN 6 INCHES IN DIAMETER SHALL BE CUT AND REINFORCED
- 3 104) WITH SHEET STEEL.
- 2 105) TOUCH UP PAINTING
- 3 106) FIELD TOUCH UP BARE SPOTS, SCRATCHES, WELDING AND SIMILAR MARRING OF
- 3 107) DECK.

6. ERECTION

***END OF SECTION

ANC
12-13



CENTEX CONSTRUCTION COMPANY, INC.

POST OFFICE BOX 427 • MERRIFIELD, VA. 22116 • TELEPHONE (703) 698-6886

PLEASE ADDRESS REPLY TO:

December 11, 1985

P.O. BOX 6510
McLEAN, VA 22106-6510
Phone: (703) 448-6810

Arthur J. Carlucci, Project Manager
New Building Project Field Office
Room 3E40, Headquarters Building
Washington, D.C. 20505

Re: CIA Headquarters Expansion
Bid Package 2
Langley, Virginia
Contract No. GS-11B-19066

Subj: Centex Claim P-61
GSA CE #62 35
Structural Steel and Deck Delay Claim

Gentlemen:

Attached for your review and consideration is a copy of Bristol Steel and Iron Works letter of December 6, 1985. This letter was previously distributed at our meeting held December 10, 1985 with Bristol Steel, Centex and the GSA.

This supplemental information further substantiates our claim. We await your review and comments.

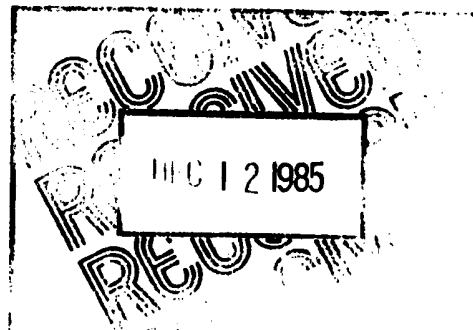
Very truly yours,

CENTEX CONSTRUCTION COMPANY, INC.

Joseph A. Pius
Assistant Project Manager

Enclosure

cc: Merrifield Office
Proposal File P-61



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RICHMOND DIVISION

1000 East Fourth Street

P.O. Box 27532

Richmond, Virginia 23261

(804) 233-7691 Telex 827350

Bristol Steel

AND IRON WORKS, INC.

December 6, 1985

①

Centex Construction Co., Inc.
P. O. Box 6510
McLean, Virginia 22106-6510

ATTN: Mr. John Milhausen

Re: CIA Headquarters Expansion
Langley, Virginia

Subj: Structural Revised and
Resubmitted Drawings

Dear Mr. Milhausen:

After reviewing the Architect's position on the reasons for requiring re-submittal on the twenty-eight structural drawings for Areas E and F, we find they fall into seven categories.

1. Shop Drawings 28, 33, 94 - The Architect clearly stated that his revised connection was a better connection than the connection we submitted in accordance with Detail 4 on Sheet 7-S3-25. The Architect did not say that the submitted connection detail would not satisfy the load requirements, he only said that his new detail was better. This is clearly a revision to his specified detail and as such is a change in the contract.
2. Drawings 96, 99, 132, 136, 139, 141 - The Architect stated that the seat plate connection detail concerned him and wanted to strengthen that connection. The result was a very expensive connection that was designed to be used only in the case of moment connection occurring at the column flanges. This was not the case on the affected columns. Again the Architect revised a specified connection because he was uncomfortable with it, and again revised the contract.
3. Drawings 24, 34, 37, 55 - The Architect's reasoning on these drawings was that resubmittal was required to show our compliance with his revisions, since he would not see the drawings unless they were resubmitted for approval. We are certain that if the Architect would have given these drawings an approval code of "2" with the provision that we send him a copy of the final shop drawing we would have been pleased to send them to him.



DEPENDABLE STRUCTURAL STEEL SERVICE SINCE 1908

Bristol Steel

AND IRON WORKS INC.

Centex Construction Co., Inc.

December 6, 1985

Page (2) of 4

4. Drawings 167, 171, 175, 208, 209, 210, 219, 288, 289, 290 - We were required to resubmit these drawings since a one-sided connection was not shown on our typical details sheets. According to Section 05100 Lines 21-22: "Submit complete shop drawings conforming with the requirements of the current AISC specifications." The one sided connection is in accordance with AISC specs. The Architect stated that our detailer chose the connection for the re-submitted drawings, so therefore, it was not a change in the contract. Our detailer was given the choice of resubmitting the drawings using a connection designed for moment connections and having it approved or to submit a one sided connection on a TD sheet for review and if approved we would then have to resubmit the detail sheets. Therefore, our detailer was given the choice of a 30 day delay before the drawings could be released to our shop or at least an additional 60 day delay. What choice did our detailer have? The specifications do not state (Section 05100 Lines 28-34) that only the detail shown on the typical detail sheets can be used, only that they must be in compliance with AISC requirements. The premise on which the Architect based his resubmittal requirement was not accurate. Even if the specifications did state that the connections shown on the typical detail sheets were the only ones to be used, the Architect could have, in the interest of expediting the project, reviewed the one-sided connections and approved the connections provided that our detailer subsequently submit a one-sided connection on a TD sheet. These columns had no moment connections on them so, requiring us to supply a connection intended to be used in conjunction with moment connections is a change in the contract.
5. Drawing 137 - We were required to resubmit this drawing due to the limitations of SH & G's computer programming, not due to any inaccuracy of our detail drawing.
6. Drawing 7, 10 - The Architect revised our submitted connection to a special connection and changed the elevation of the angle. The Architect should have shown this special connection on the contract drawings and any change in the elevations shown on the contract will affect the connections and is a change in the contract.
7. Drawing 61, 19 - These two drawings reflect similar conditions to item 3 and 6. Surely we could be trusted to eliminate the blockouts on Sheet 65 and incorporate the blockout shown on drawing 19. The other two pieces on Sheet 19 were revised due to a change in the required elevation, which is a change to the contract.

Centex Construction Co., Inc.
 December 6, 1985
 Page ③ of 4

Bristol Steel
 AND IRON WORKS INC.

When an Architect shows specific connections on the contract drawings, the connections specified are requirements not merely suggestions. Our estimating and bidding processes rely heavily on the connection details shown on the contract drawings since the depicted connections are the connections to be used on the project. When these specified connections are revised to a connection that is considerably more expensive to fabricate and to detail, we are entitled to reimbursement for this unanticipated expense.

The Architect claimed that he attempted to expedite the job, but also required resubmittal of drawings to show compliance. This is a contradiction, if an Architect is trying to expedite a project, there are alternatives to requiring shop drawings to be resubmitted to prove that we had incorporated his revisions (i.e., eliminating blockouts, deleting a pattern of 4 holes, adding a second line of bolts, adding a stiffener, etc.) into our drawings.

* At the conclusion of our meeting about these drawings a representative of the Architectural firm, Mr. Everett Medling, informed me that SH & G had revised their review process. Currently, if a submittal receives an approval code of 3 by a reviewer, the drawing is sent to Mr. Medling to review. If Mr. Medling believes the drawing should have received an approval code of 2, it is changed to a 2 and returned to the submitter. If Mr. Medling agrees with the approval code 3, he and the original reviewer discuss what information is required to give the drawing an approval code of 2, then the submitter is contacted, the information, if obtainable, is given and the drawing receive approval code 2 and is returned. We believe this revision to their review process will greatly expedite the balance of the job and that SH & G should be commended for implementing it. However, this review revision occurred too late for Bristol Steel to benefit from it and clearly shows that the original review process was flawed, and, since Bristol Steel submitted a majority of the drawings reviewed under the original process, we suffered most from the flawed review process and Bristol Steel was the primary cause for revising the review process which is expediting the work of all of the other subcontractors on site.

The beams affected by the revision in item 4 were not required to be resubmitted by the Architect. This is another contradiction. If the Architect trusts Bristol Steel to stop production of material in the shop, revise the connections and fabricate the beams in accordance with the new connection without resubmitting the drawings for approval, then we cannot understand resubmittal to show compliance.

Since the TD drawings were a requirement of this project, we assume they were to expedite approval. Also, since the TD drawings were approved by the Architect, why did we have to resubmit a drawing for approval when the Architect directed us to utilize a connection that had already been approved?

Bristol Steel
AND IRON WORKS INC.

Centex Construction Co., Inc.
December 6, 1985
Page ④ of 4

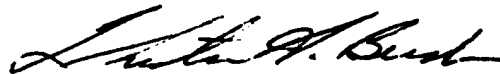
Bristol Steel cannot be expected to anticipate, prior to bidding a project, which connections the Architect is going to "improve" during the approval process, nor can we anticipate being required to provide very expensive moment connections where, according to the contract documents, none are needed.

We have not altered our original position which was that the drawings in question were unnecessarily resubmitted for approval. This resubmission adversely affected both our fabrication and orderly erection sequence severely. Particularly, in light of, the production schedule required, of which the Architect was well aware and should have realized the impact these resubmittals would cause.

Bristol Steel appreciates the efforts of the GSA to expedite this project, without their help our progress would have been delayed by the Architect for 127 days rather than the 63 days in our claim. In fact, Bristol Steel believes that had the GSA performed the approval of our shop drawings these delays would not have occurred.

We have also included a copy of our detailer's reaction to these resubmitted drawings.

Sincerely,



Chester A. Bush
Contracting Engineer

CAB/ce

cc: Mr. Roy Daniel

enclosure

SH&G 13155 BP-2
CENTRAL INTELLIGENCE AGENCY
HEADQUARTERS EXPANSION
910 PACKAGE 2
HEADQUARTERS BLDG & VISITORS CENTER

SECTION 05100 PAGE 1
DATE 06/13/84 TIME 17.713
STRUCTURAL STEEL

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1)	1. GENERAL PROVISIONS
2)	2) EXAMINATION AND ACCEPTANCE OF WORK IN PLACE
3)	3) EXAMINE WORK IN PLACE ON WHICH THIS WORK IS DEPENDENT. DEFECTS WHICH
4)	4) MAY INFLUENCE SATISFACTORY COMPLETION AND PERFORMANCE OF THIS WORK
5)	5) SHALL BE CORRECTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE
6)	6) APPLICABLE SECTION OF WORK PRIOR TO COMMENCEMENT OF THE WORK.
7)	7) COMMENCEMENT SHALL BE CONSTRUED AS WORK IN PLACE BEING ACCEPTABLE FOR
8)	8) SATISFYING THE REQUIREMENTS OF THIS SECTION.
9)	9) REFERENCE SPECIFICATIONS
10)	10) THE WORK SHALL COMPLY WITH REQUIREMENTS OF THE FOLLOWING STANDARD
11)	11) SPECIFICATIONS, UNLESS OTHERWISE SPECIFIED:
12)	12) AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF
13)	13) STRUCTURAL STEEL FOR BUILDINGS.
14)	14) AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
15)	15) SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.
16)	16) AWS STRUCTURAL WELDING CODE, D1.1.
17)	17) SSPC PA1 PAINT APPLICATION SPECIFICATIONS AND APPLICABLE SSPC
18)	18) SURFACE PREPARATION SPECIFICATIONS SSPC-SP3 POWER TOOL CLEANING.
19)	2. SUBMITTALS
20)	20) SHOP DRAWINGS
21)	21) SUBMIT COMPLETE SHOP DRAWINGS CONFORMING WITH THE REQUIREMENTS OF THE
22)	22) CURRENT AISC SPECIFICATIONS. SHOP DRAWINGS SHALL INCLUDE COMPLETE
23)	23) INFORMATION FOR FABRICATION AND ERECTION OF COMPONENT PARTS OF THE
24)	24) STRUCTURE AND SHALL SHOW LOCATION, TYPE AND SIZE OF BOLTS AND WELDS.
25)	25) COORDINATE APPLICABLE ITEMS WITH PRECAST ARCHITECTURAL CONCRETE SHOP
26)	26) DRAWINGS. PREPARATION OF SHOP DRAWINGS SHALL NOT BE SUBLET WITHOUT THE
27)	27) WRITTEN PERMISSION OF THE CONTRACTING OFFICER.

SECTION 05100 PAGE 2
DATE 06/13/84 TIME 17.713
STRUCTURAL STEEL

SH&G 13155 UP-2
CENTRAL INTELLIGENCE AGENCY
HEADQUARTERS EXPANSION
BID PACKAGE 2
HEADQUARTERS BLDG & VISITORS CENTER

(6)

- 3 28) AFTER AWARD OF CONTRACT, BUT PRIOR TO THE BEGINNING OF DETAILED SHOP
29) DRAWINGS, SUBMIT DRAWINGS SHOWING TYPICAL DETAILS OF CONNECTIONS. THE
30) CONTRACTOR SHALL ARRANGE TO MEET WITH THE CONTRACTING OFFICER AT THE
31) OFFICE OF THE CONTRACTING OFFICER APPROXIMATELY ONE WEEK AFTER
32) SUBMITTAL TO REVIEW DRAWINGS AND COORDINATE COMMENTS. THE TYPICAL
33) DETAILS AS ACCEPTED SHALL BE USED AS THE BASIS FOR DEVELOPING DETAILED
34) SHOP DRAWINGS.
- 2 35) WELDING PROCEDURES
- 3 36) SUBMIT WELDING PROCEDURES CONFORMING TO AWS D1.1, STRUCTURAL WELDING
37) CODE, TO THE CONTRACTING OFFICER AND INSPECTION AGENCY.
- 4 38) PREQUALIFIED WELDING PROCEDURES SHALL BE SUBMITTED AS WRITTEN
39) PROCEDURE SPECIFICATIONS.
- 4 40) WELDING PROCEDURES WHICH ARE NOT PREQUALIFIED SHALL BE QUALIFIED BY
41) TEST(S) AND APPROVED BY THE CONTRACTING OFFICER PRIOR TO THEIR USE
42) ON THE PROJECT.
- 4 43) TEN DAYS AFTER SUBMITTAL OF THE WELDING PROCEDURES, AND PRIOR TO
44) START OF FABRICATION, THE CONTRACTOR SHALL ARRANGE TO MEET WITH THE
45) CONTRACTING OFFICER, AND THE INSPECTION AGENCY AT THE CONTRACTOR'S
46) FABRICATION PLANT TO REVIEW THE WELDING PROCEDURES SUBMITTED.
- 3 47) APPROVED PROCEDURES FOR WELDING SHALL BE ENFORCED AND SUPERVISED,
48) DURING FABRICATION AND ERECTION OF STRUCTURAL STEEL, BY EXPERIENCED
49) SUPERVISORS KNOWLEDGEABLE OF GOOD WELDING PRACTICES.
- 4 50) COOPERATE WITH THE INSPECTION AGENCY, WHO SHALL EXAMINE THE WORK,
51) TO MAKE CERTAIN THAT ONLY APPROVED WELDING PROCEDURES ARE EMPLOYED.
- 2 52) CERTIFICATES OF COMPLIANCE
- 3 53) SUBMIT CERTIFICATE OF COMPLIANCE FOR THE FOLLOWING:
- 4 54) PAINT
- 4 55) WELDING ELECTRODES AND RODS
- 2 56) QUALIFICATION OF WELDERS
- 3 57) WELDERS, WELDING OPERATORS AND TACKERS SHALL BE QUALIFIED BY AN
58) INDEPENDENT INSPECTION (AND TESTING) AGENCY, IN ACCORDANCE WITH AWS
59) D1.1 STRUCTURAL WELDING CODE.
- 4 60) ANY WELDER, WELDING OPERATOR OR TACKER FOUND TO BE PRODUCING
61) UNSATISFACTORY WORK, EVEN THOUGH HE OR SHE HAS PASSED QUALIFICATION
62) TESTS, SHALL BE IMMEDIATELY REQUALIFIED OR REPLACED WITH QUALIFIED
63) WELDER.
- 3 64) SUBMIT TO THE INSPECTION (TESTING) AGENCY AND THE CONTRACTING OFFICER
65) (OPTIONAL) THE NAMES OF WELDERS, WELDING OPERATORS AND TACKERS TO BE
66) EMPLOYED ON THE WORK TOGETHER WITH THEIR WELDING CERTIFICATES STATING
67) THE TYPES OF WELDING AND POSITIONS FOR WHICH THEY ARE QUALIFIED, THE
68) CODES AND PROCEDURES UNDER WHICH THEY ARE QUALIFIED, THE DATES OF
69) QUALIFICATION AND THE TESTING AGENCY CERTIFYING THE QUALIFICATION
70) TESTS. THE CERTIFICATES SHALL BE KEPT CURRENT AND FILED WITH THE
71) INSPECTION (TESTING) AGENCY FOR THE DURATION OF THE CONTRACT.
- 3 72) THE INSPECTION (TESTING) AGENCY SHALL CHECK THE WELDING CERTIFICATES
73) FOR CONFORMANCE WITH AWS D1.1 AND THIS SECTION OF THE SPECIFICATIONS.
- 3 74) REQUALIFICATION TESTS WHEN REQUIRED SHALL BE MADE AT THE CONTRACTOR'S
75) EXPENSE.
- 2 76) MILL TEST REPORT

PSD

PALMETTO STEEL DETAILERS, INC.

Structural Steel Detailers

536 MEETING STREET • WEST COLUMBIA, S.C. 29169 • (803) 796-6399

(7)

November 11, 1985

Bristol Steel & Iron Works, Inc.
Post Office Box 27532
Richmond, Virginia 23261
ATTN: Chet Bush

NOV 14 1985
BRISTOL STEEL
& IRON WORKS, INC.

RE: C.I.A. - Headquarters
Building Expansion
Langley, Va.
Contract No.: 1814

Dear Chet,

I hope this letter will finally put to rest our many discussions concerning "Why" drawings needed to be resubmitted for approval. I would like to state at the beginning that we have had no problems communicating with Mr. Angeles and Mr. Sabensky of Smith, Hinchman & Grylls, Associates. The questions we have had to ask were responded to in a prompt and concise manner. As always there are some areas on a project where we as detailers have to make some assumptions in order to keep the project on schedule and avoid delays. Some resubmittals were caused by our assumptions and some were caused directly by design changes on approval. I have attached a list of drawings in areas "E" and "F" with a brief description of what the approval changes were and why. I would also like to take the time to give my response to the approval changes.

I feel that when the engineer is approving drawings and actually takes the time to show what is required then "resubmittal" should not be required. The following sheets fall into this category: 7, 10, 19, 24, 28, 33, 34, 37, 55, 65, 94, 137.

Changes concerning Detail "2" on Design Drawing 7-S4-3 was a design revision. Detail "2" called for a seated connection to support beam framing to column web which is the way it was detailed. This change not only affected columns and seated beams but changed beams framing to column flanges due to stiffeners being added from flange to flange. The following beam end connections were changed: 37B1, B2, 81B3, B4, B1, B2, 96B1, 90B2, B3, 99B3, B4 and 138B1.

The following sheets were revised and resubmitted due to this change: 96, 99, 132, 136, 139, 141.

The following change concerns the 2" offset at columns, beams were detailed with one-sided connections per AISC. This was not covered by a "TD-Sheet", but we felt this would be acceptable. The detail that was used (similar to Type #3 on TD-4) was actually set up for columns with moment connected beams framing to column flanges

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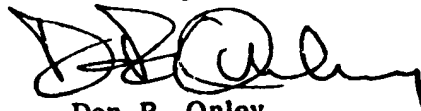
which was not the case on these columns. The following is a list of beams that were released for shop that we had to place under a "Hold" and have returned to us for correction and re-checking: 63B1, 51B5, 41B3 R/L, 41B2 R/L, 45B1 R/L, 101B5 R/L, 33B3, 36B1 R/L, 61B1 R/L, 36B4, 36B2, 26B4, 105B1.

The following is a list of sheets that had to be revised and resubmitted for approval: 167, 171, 175, 208, 209, 210, 219, 288, 289, 290.

In closing, I feel that changes to sheets 7, 10, 19, 24, 28, 33, 34, 37, 55, 65, 94 and 137 were minor and in no way affected the structural integrity of the building and therefore should not have required resubmission. The balance of sheets that were resubmitted were all covered by our "typical detail sheets" and although beams connecting to these were Code "2" on approval, columns were noted Code "3" and had to be resubmitted. I feel that if the engineer trusted us to change beams without "resubmission" then columns should have required same consideration to avoid untimely delays on project.

If you have any other questions or need addition information concerning the "resubmissions", please feel free to call.

Sincerely,



Don R. Onley

DRO/jdn

④
SHEET #:

- 7 Dimensions and elevations in Sections 3 and 4 shown on Design Drawing 7-S4-8 did not agree with floor framing shown on Design Drawing 7-S3-14 and Detail "1" on 7-S3-30. We changed sections 3 and 4 on 7-S4-8 to make framing match and asked for verification on approval. Engineer revised sections on approval.
- 10 Same as Sheet #7.
- 19 Section "4" on Design Drawing 7-S3-25, at column lines "17a" and "10b" we had extended Beam framing in North-South direction past column and blocked haunch connection on column to clear beam flange. On approval engineer reversed what we had done (cut and chipped extended beam, removed block from haunch connection.)
- 24 On Design Drawing 7-S3-5 at Column Grid E-12 and E-15, we added beam in East-West direction, like the beam shown in Detail "20" on 7-S4-9 at Grid C-15. We felt beam was required and had been left off by engineer because of all similar conditions at this floor, but engineer noted on approval beam was not required.
- 28 Design Drawing 7-S3-25 at Column Grids E2-10b and E2-17a, we had provided "Haunch Connection" (Detail 4 on 7-S3-25) for beam framing in North-South direction and blocked beam framing in East-West to clear "Haunch Connection" and framed it to column. On approval engineer removed haunch connection and seated beam in North-South direction on top of beam in East-West direction.
- 33 Same as Sheet #28.
- 34 Engineer added (2) bolts to Detail 10 on Drawing TD-4. Sheet number 34 was submitted for approval before TD4 was received back from approval.
- 37 Engineer added stiff. plates to beam web on detail 9 - Drawing TD-4. Sheet #37 was submitted for approval before TD4 was received back from approval
- 55 Engineer added (2) bolts to seat connected beam on right end giving a total of (4) bolts (no beam opposing) although only (2) bolts used on left end (beam opposing).
- 65 Column referred to on Sheet #19.
- 94 Column referred to on Sheet #28.
- 96 Beam end connection revised due to change at column web, originally had seat connections at opposite side of column web as shown in Detail 2 - Drawing 7-S4-3 but was changed to Type "3" connection as shown on TD4, had to revise bolt spacing in clip angle.

(16)

- 99 Revised beam end connections from seated type as shown in Detail "2" on Drawing 7-S4-3 to plate type connection as shown on Drawing TD4 - Detail 3. Changes made on approval.
- 132 Column affected by change noted on Sheet #99.
- 136 Same as Sheet #132.
- 137 Engineer questioned shear connection on end of beam, beam was detailed in accordance with engineer's detail 9 on Drawing 7-S4-12. No change made to beam.
- 139 Same as Sheet #132.
- 141 Same as Sheet #132.
- 167 Beams framing to these columns were offset column centerline 2", standard double angle connections could not be used. We had used one-sided connections per AISC 8th edition page 4-84. When beams were received from approval, the ends were encircled and noted "use 2 angles" and drawings were marked Code "2". We felt at this time that beams had not been matched to columns and approver had not seen the 2" offset, beams were not changed and were released to shop as is. Later, we received columns from approval and the connections in the web were encircled and noted "use 2-angle conn." and sheets were marked Code "3". At this time I called Mr. Angeles to discuss what we had done. He felt that a one-sided connection was not permissible because it had not been covered by a "TD-Sheet" and that we should use a connection similar to Type #3 on Sheet #TD-4. This involved changing beams and columns
- 171 Same as sheet #167.
- 175 Same as Sheet #167.
- 208 Same as Sheet #167.
- 209 Same as Sheet #167.
- 210 Same as Sheet #167.
- 219 Same as Sheet #167.
- 288 Same as Sheet #167.
- 289 Same as Sheet #167.
- 290 Same as Sheet #167.

	5/20/65	5/21/65	5/22/65	5/23/65	5/24/65	5/25/65	5/26/65	5/27/65	5/28/65	5/29/65	5/30/65	5/31/65	6/1/65	6/2/65	6/3/65	6/4/65	6/5/65	6/6/65	6/7/65	6/8/65	6/9/65	6/10/65	6/11/65	6/12/65	6/13/65	6/14/65	6/15/65	6/16/65	6/17/65	6/18/65	6/19/65	6/20/65	6/21/65	6/22/65	6/23/65	6/24/65	6/25/65	6/26/65	
Fabrication Delay (R+R*)																																							
Deck Delay (R+R*)																																							
Field Delay (R+R*)																																							
Rain Delay																																							





TOTALS

Fabrication Delay 16 DAYS
 DECK Delay (DEL.) 20 DAYS
 Field Delay (LOADS & RES) 27 DAYS
 Rain Delay 4 DAYS
 Change Order CE # 11 3 DAYS
 Change Order CE # 14 9 DAYS
79 DAYS

ONLY FOR
E & F

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6/27/65
6/28/65
6/29/65
6/30/65
7/1/65
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7/3/65
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8/3/65

Fabrication Delay (R+R*)	
Dock Delay (R+R*)	
Field Delay (R+R*)	
Rain Delays	

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Revised And Resubmitted Drawings

8/4/65
8/5/65
8/6/65
8/7/65
8/8/65
8/9/65

Fabrication
Delay
(R+R)

Deck
Delay
(R+R)

Field
Delay
(R+R)

PAin
Delay

